ST. ANDREWS EPISCOPAL CHURCH

MOREHEAD CITY, NORTH CAROLINA

DRAWING LIST

Coastal Architecture

Coastal Architecture PLLC

- Architectural Design
- Planning
- Interiors

AIA

Member of the American Institute of Architects

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MOREHEAD CITY, NORTH CAROLINA

COASTAL ARCHITECTURE pllc — REGISTERED ARCHITECTURAL CORPORATION — NORTH CAROLINA — CERT. NO. 51792 — MOREHEAD CITY, NC

LEE D. DIXON, JR. — REGISTERED ARCHITECT — NORTH CAROLINA — 6419 — MOREHEAD CITY, N.C.

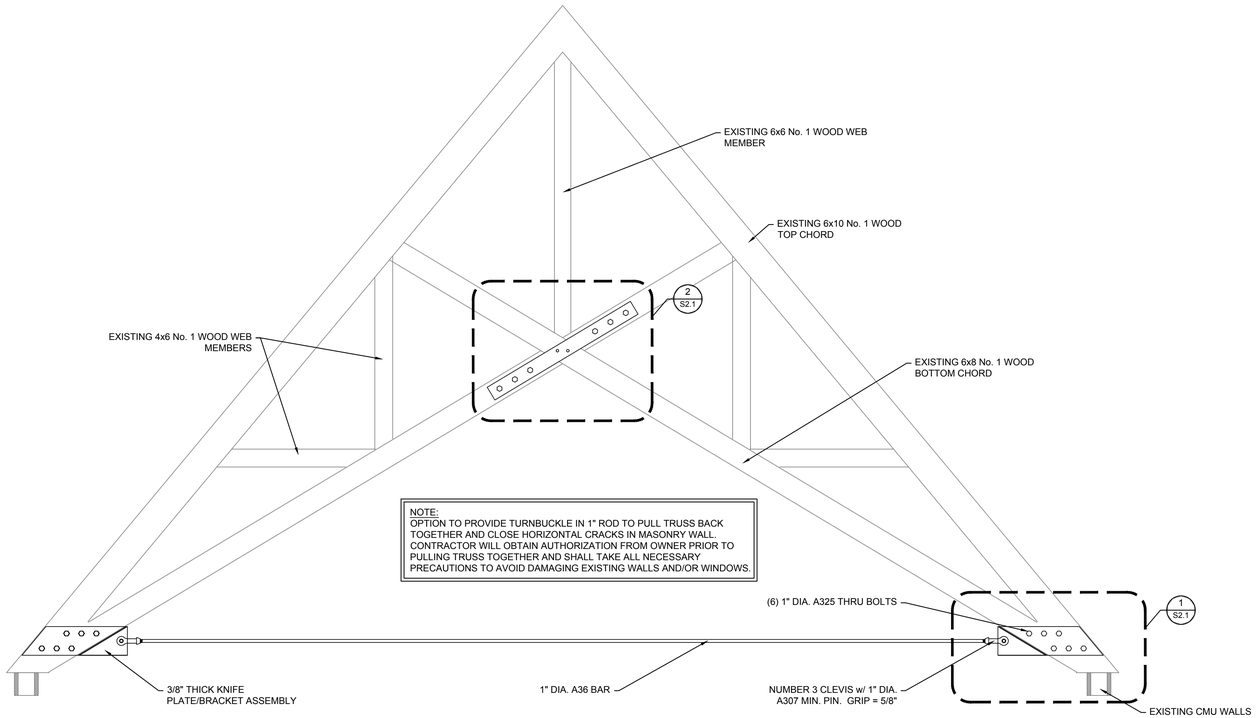
COVER SHEET

17018

ISSUED:	4/9/18
DWG BY:	LSA
CKD BY:	LDD

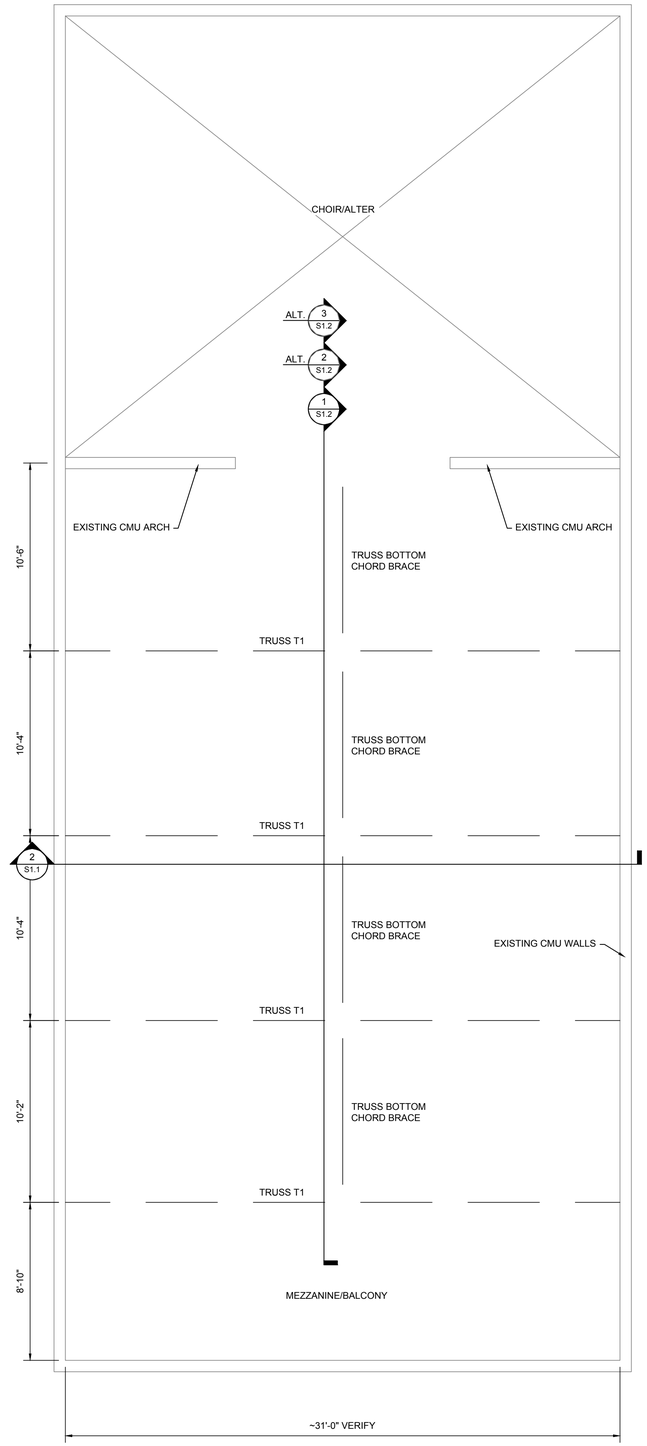
REVISIONS

SHEET NO.
CS-1
OF



2 T1 TRUSS PROFILE

SCALE: 1/2" = 1'-0"



1 SANCTUARY LAYOUT

SCALE: 1/4" = 1'-0"

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PRELIMINARY - NOT FOR CONSTRUCTION

Project Name

ST. ANDREWS EPISCOPAL CHURCH

MOREHEAD CITY, NORTH CAROLINA

Sheet Title

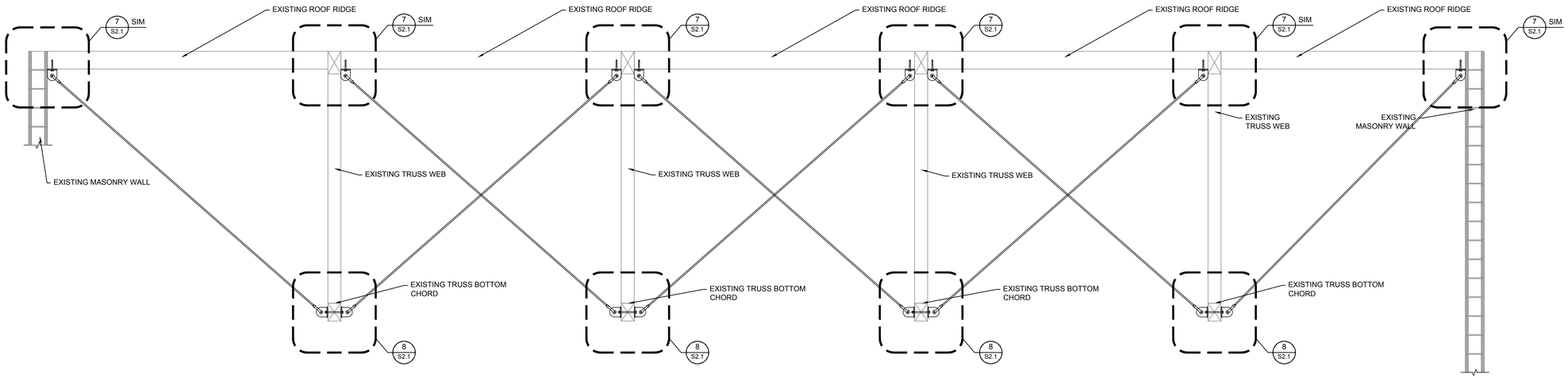
GENERAL NOTES AND FRAMING LAYOUT

DESIGNED BY:	KJK
DRAWN BY:	KJK
APPROVED BY:	JMF
PROJECT #:	17-201
DATE:	03/27/2018

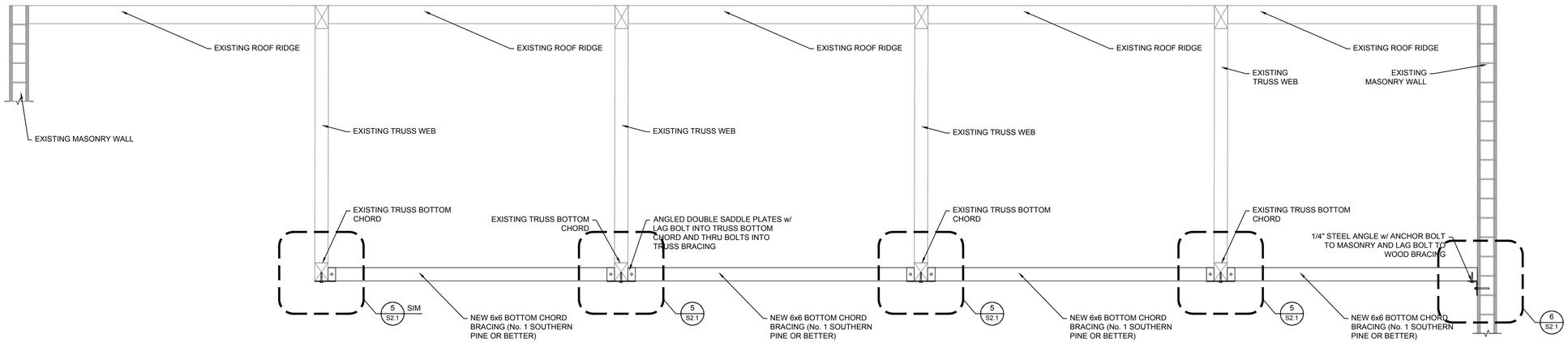
No.	Revision	Date

Sheet

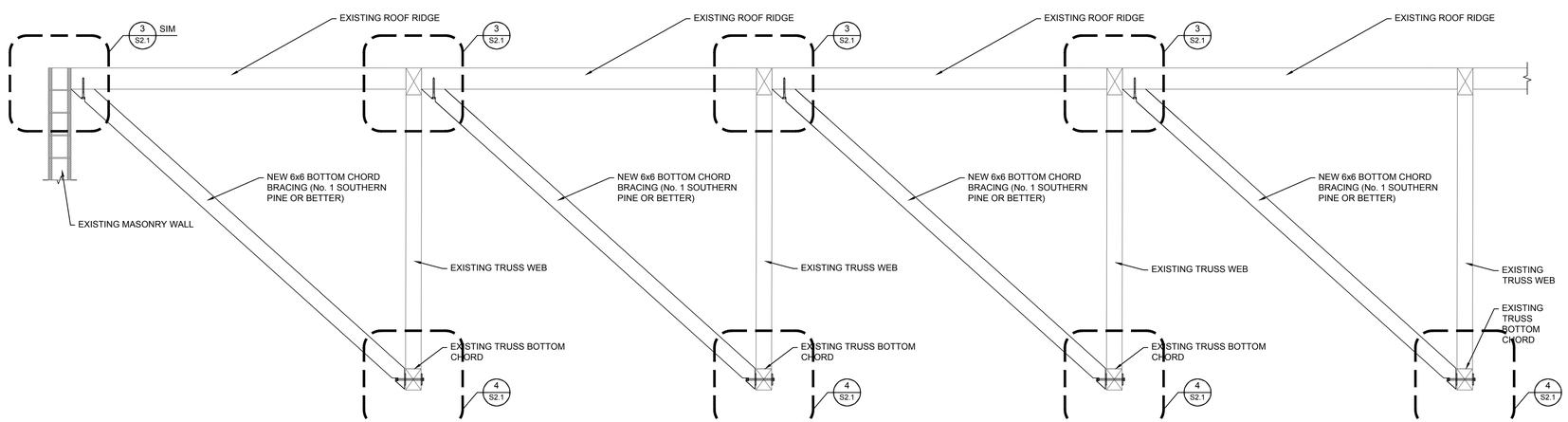
S1.1



1 DIAGONAL ROD BRACING SECTION
SCALE: 1/2" = 1'-0"



2 ALTERNATE 1: HORIZONTAL BRACING SECTION
NOTE:
SECTION IS AN ALTERNATE TO BRACING DETAIL 1/S1.2.
SCALE: 1/2" = 1'-0"



3 ALTERNATE 2: DIAGONAL BRACING SECTION
NOTE:
SECTION IS AN ALTERNATE TO BRACING DETAIL 1/S1.2.
SCALE: 1/2" = 1'-0"

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HORIZONTAL
BRACE SECTION

DESIGNED BY:	KJK
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S1.2

STRUCTURAL NOTES

GENERAL NOTES:

1. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

2. ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD AND WITH ALL OTHER DRAWINGS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

3. THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING (AND ACCOMPANYING FOOTINGS), GUYS OR TIEDOWNS.

4. ADDITIONAL OBSERVATIONS AS A RESULT OF REJECTION OF WORK COMPLETED AND/OR ADDITIONAL OBSERVATIONS DUE TO THE DEFICIENCIES IN WORK OBSERVED WILL BE AT THE EXPENSE OF THE CONTRACTOR.

5. ALL STRUCTURAL SHOP DRAWINGS TO BE REVIEWED BY JOB SUPERINTENDENT IN ADDITION TO ALL PERSONNEL DEEMED NECESSARY BY CONTRACTOR PRIOR TO SUBMITTAL TO ENGINEER FOR APPROVAL.

6. ALL SHOP DRAWING RESUBMITTALS SHALL INCLUDE A WRITTEN DETAILED LIST OF LOCATIONS AND DESCRIPTIONS OF ALL CHANGES MADE FROM PREVIOUS SUBMITTAL. LIST SHALL BE SPECIFIC AND GENERAL NOTES SUCH AS 'DIMENSIONS CORRECTED' ARE NOT ACCEPTABLE.

DESIGN CODES:

2012 NORTH CAROLINA STATE BUILDING CODE.

AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN.

2005 NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION

DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED WITH THE FOLLOWING SUPERIMPOSED LOADINGS:

ROOF:		
GROUND SNOW LOAD,	Pg =	10 psf
DESIGN ROOF SNOW LOAD,	Pf =	5 psf
SNOW EXPOSURE FACTOR,	Ce =	1.0
SNOW LOAD IMPORTANCE FACTOR,	Is =	1.1
THERMAL FACTOR,	Ct =	1.0
ROOF LIVE LOAD	20 psf	
WIND:		
BASIC WIND SPEED (3 SEC GUST)	132 mph	
EXPOSURE CATEGORY	C	
IMPORTANCE FACTOR	1.15	
WIND BASE SHEARS,	Vx =	17.7k
	Vy =	50.6k

STRUCTURAL STEEL:

1. STEEL SHALL CONFORM TO ASTM A992 (Fy=50 ksi) FOR ALL W-SHAPES, AND ASTM A36 (Fy=36 ksi) FOR ALL OTHER MISCELLANEOUS SHAPES AND PLATES. STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B (Fy=46 ksi). STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, GRADE B, TYPE "E" OR "S" (Fy=42 ksi).

2. STEEL SHALL CONFORM TO THE LATEST EDITION OF "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC (AISC).

3. ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR SHALL BE HOT-DIPPED GALVANIZED.

4. ALL SHOP CONNECTIONS TO BE WELDED (UTLIZING E70XX ELECTRODES) AND FIELD CONNECTIONS TO BE BOLTED, UNLESS OTHERWISE NOTED. STEEL TO RECEIVE ONE SHOP COAT AND ONE FIELD TOUCH UP COAT OF APPROVED PAINT, EXCEPT WHERE GALVANIZED IS INDICATED ON THE DRAWINGS.

5. WELDS FOR ALL EXPOSED STRUCTURAL STEEL SHALL BE GROUND SMOOTH UNLESS NOTED OTHERWISE.

6. ALL BOLTED CONNECTIONS SHALL CONSIST OF 3/4" DIAMETER (MIN.) ASTM A325 HIGH STRENGTH BOTLTS, UNLESS NOTED OTHERWISE. BEAM CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR TO SUPPORT AN END REACTION OF Wc/2, KIPS IN ACCORDANCE WITH PART 2 - "BEAM AND GIRDER DESIGN" OF THE MANUAL OF STEEL CONSTRUCTION (9th EDITION), BUT CONNECTIONS SHALL NOT HAVE LESS THAN 2 ROWS OF BOLTS. SEE ALSO DOUBLE ANGLE AND SHEAR TAB CONNECTION SCHEDULE(S) WHERE APPLICABLE.

WOOD:

1. STRUCTURAL 6x WOOD COMPONENTS HAVE BEEN DESIGNED AS SOUTHERN YELLOW PINE (SYP) OR HEM-FIR (HF) NO. 1 OR BETTER AND SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE FIBER STRESSES AND PROPERTIES:

MODULAS OF ELASTICITY (E)	1,500,000 PSI
BENDING (Fb)	1350 PSI
SHEAR (Fv)	165 PSI

2. WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109.

3. MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED OTHERWISE.

4. BOLTS IN WOOD ARE MACHINE BOLTS, UNLESS OTHERWISE NOTED. MACHINE BOLTS SHALL HAVE A SHANK DIAMETER WITHIN 1/64" OF THAT SPECIFIED. BOLTS ARE ASTM 307 STEEL. BOLT HOLES IN WOOD SHALL BE 1/32" OVERSIZE. WHERE STEEL IS CONNECTED TO WOOD, HOLES IN STEEL SHALL BE 1/16" OVERSIZE. PROVIDE STANDARD CUT WASHERS UNDER HEAD AND NUT WHERE BEARING IS AGAINST WOOD. WHERE STEEL SIDE PLATES ARE USED FOR CONNECTION, THE PLATE SHALL BE USED AS A TEMPLATE.

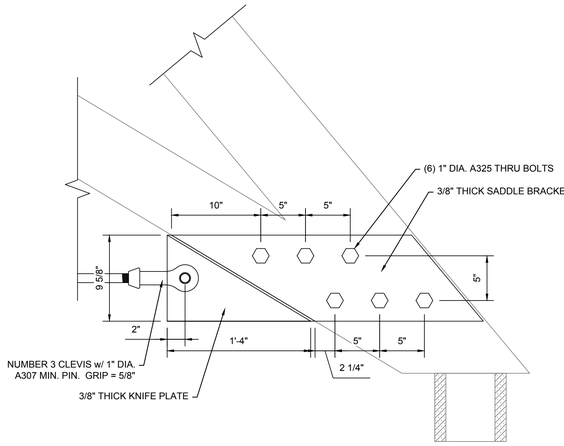
5. ALL WOOD ELEMENTS SHALL BE ATTACHED PER THE FASTENING SCHEDULE OF THE 2012 NCSBC (TABLE 2304.9.1) UNLESS OTHERWISE NOTED.

6. SEE ARCHITECTURAL DRAWINGS FOR WEATHER PROTECTION OF ALL EXPOSED WOOD MEMBERS.

WOOD FRAMING CONNECTORS:

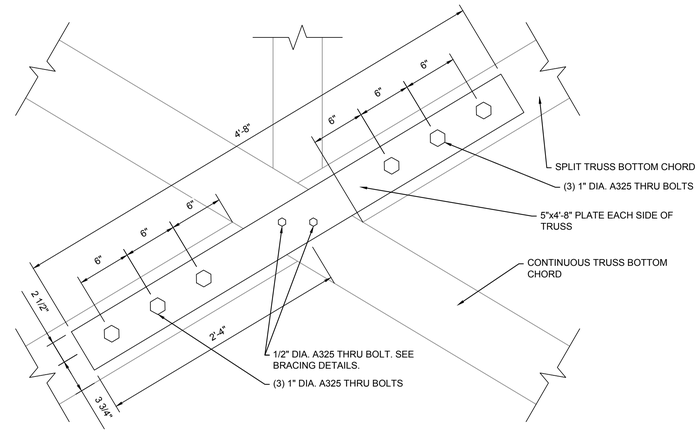
1. CONNECTOR MODEL NUMBERS SHOWN ARE "Strong-Tie" CONNECTORS AS MANUFACTURERED BY "SIMPSON Strong-Tie Co.", 1450 DOOLITTLE DR., PO BOX 1568, SAN LEANDRO, CA 94577. SUBSTITUTIONS ARE ACCEPTABLE ONLY WITH THE APPROVAL OF THE STRUCTURAL ENGINEER.

2. ALL CONNECTORS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM-A653. CONNECTORS IN CONTACT WITH PRESSURE TREATED MATERIALS SHALL HAVE G-185 COATING. CONNECTORS NOT IN CONTACT WITH TREATED MATERIALS SHALL HAVE STANDARD G-60 COATING.



SIDE FACE

1 TENSION ROD ANCHORAGE
SCALE: 1-1/2" = 1'-0"



2 SPLIT BOTTOM CHORD REINFORCEMENT
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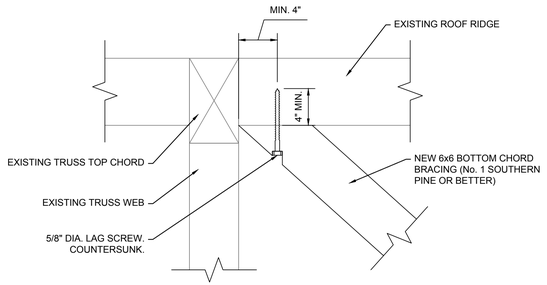

3 DIAGONAL BRACE ROOF RIDGE CONNECTION
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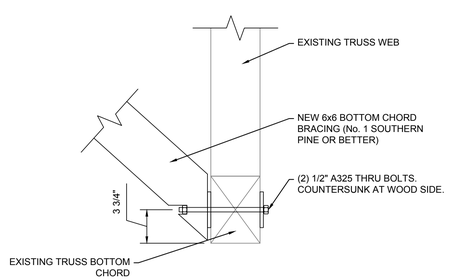

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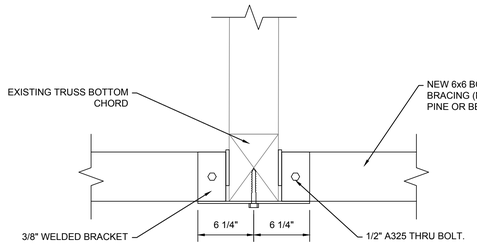
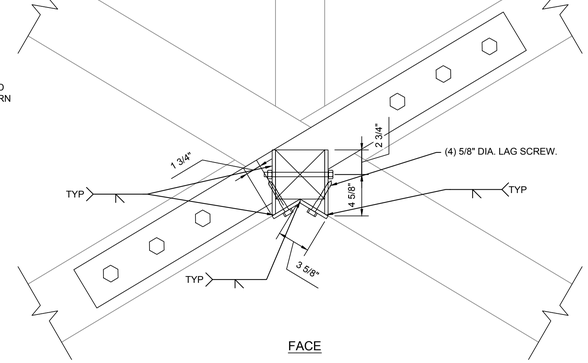


SIDE



FACE

5 HORIZONTAL BRACE BOTTOM CHORD CONNECTION
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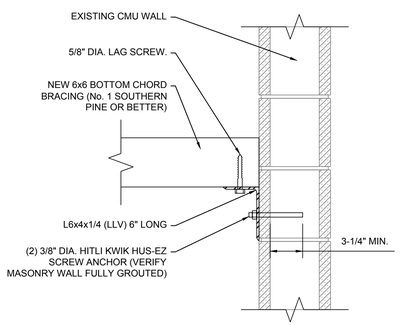
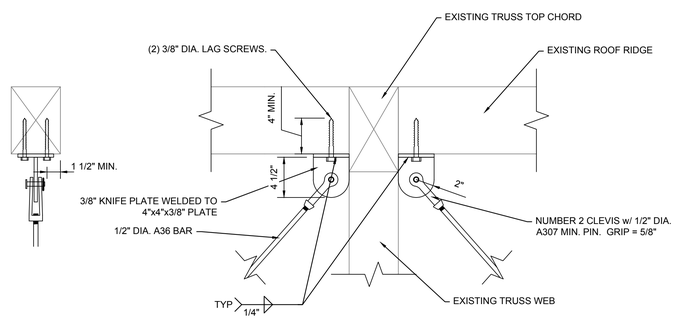


6 HORIZONTAL BRACE CONNECTION TO CMU
SCALE: 1-1/2" = 1'-0"



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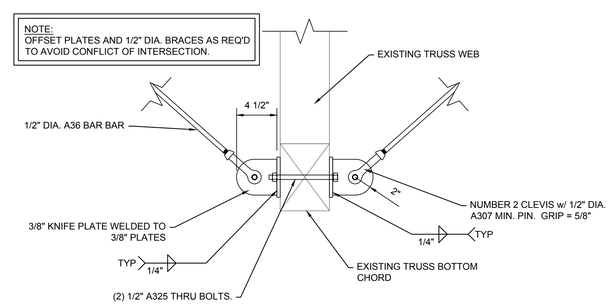
SCALE: 1-1/2" = 1'-0"



8 DIAGONAL ROD BOTTOM CHORD CONNECTION
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